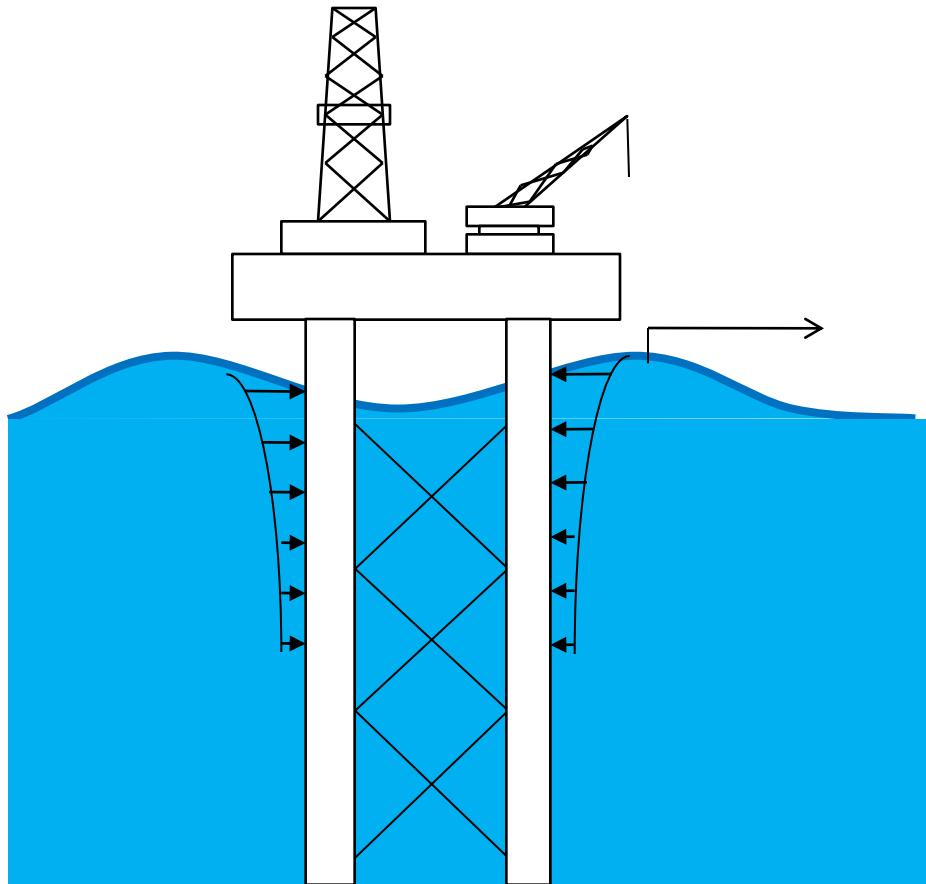
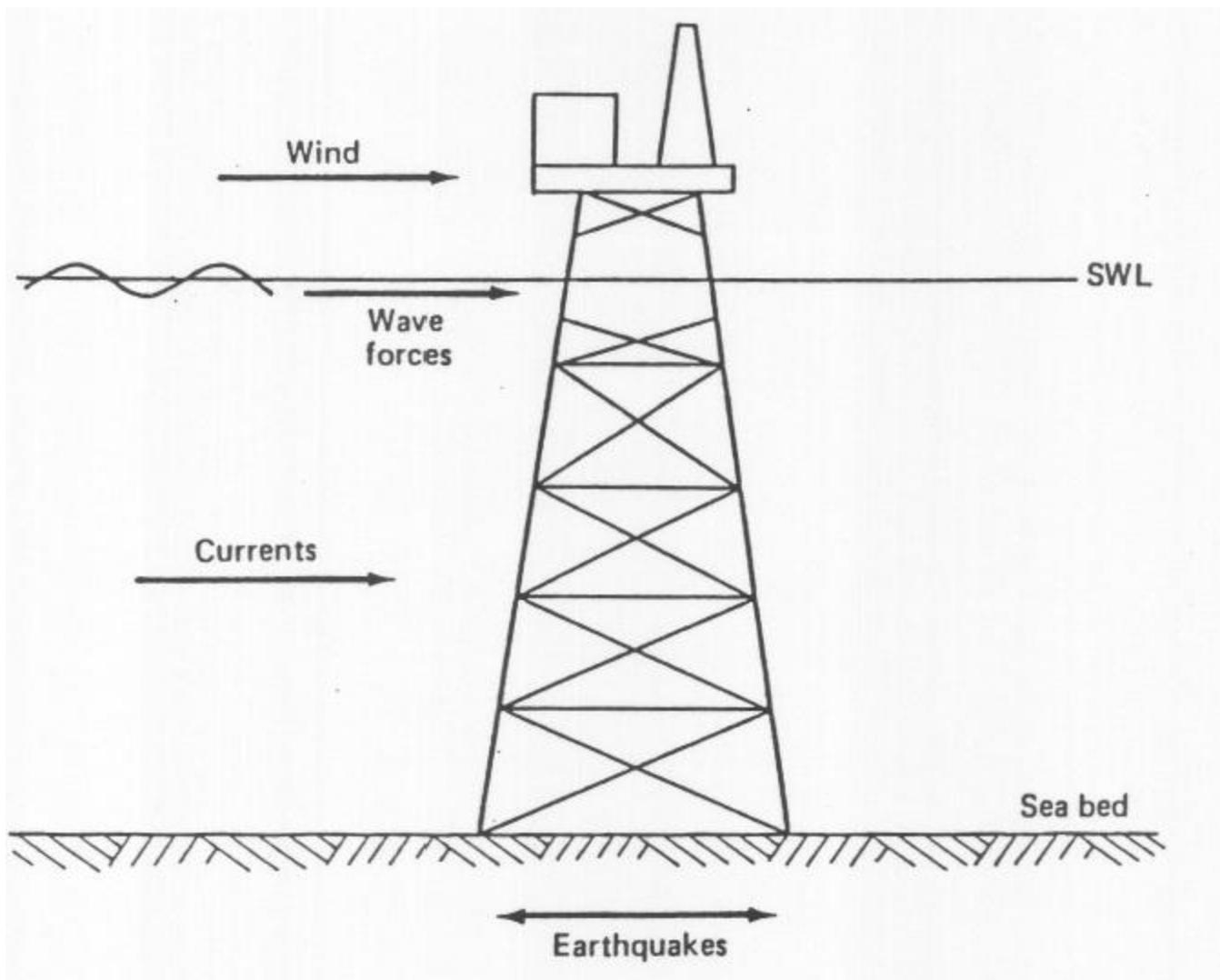


Wave, current, and wind forces



An offshore structure is designed to withstand the 100-year storm (wave/current/wind). A mono-chromatic wave of height H_{\max} is assumed.



OFFSHORE PLATFORMS are comprised of many cylindrical or prismatic components (structural elements, floatation parts, risers, tendons, mooring lines, etc)

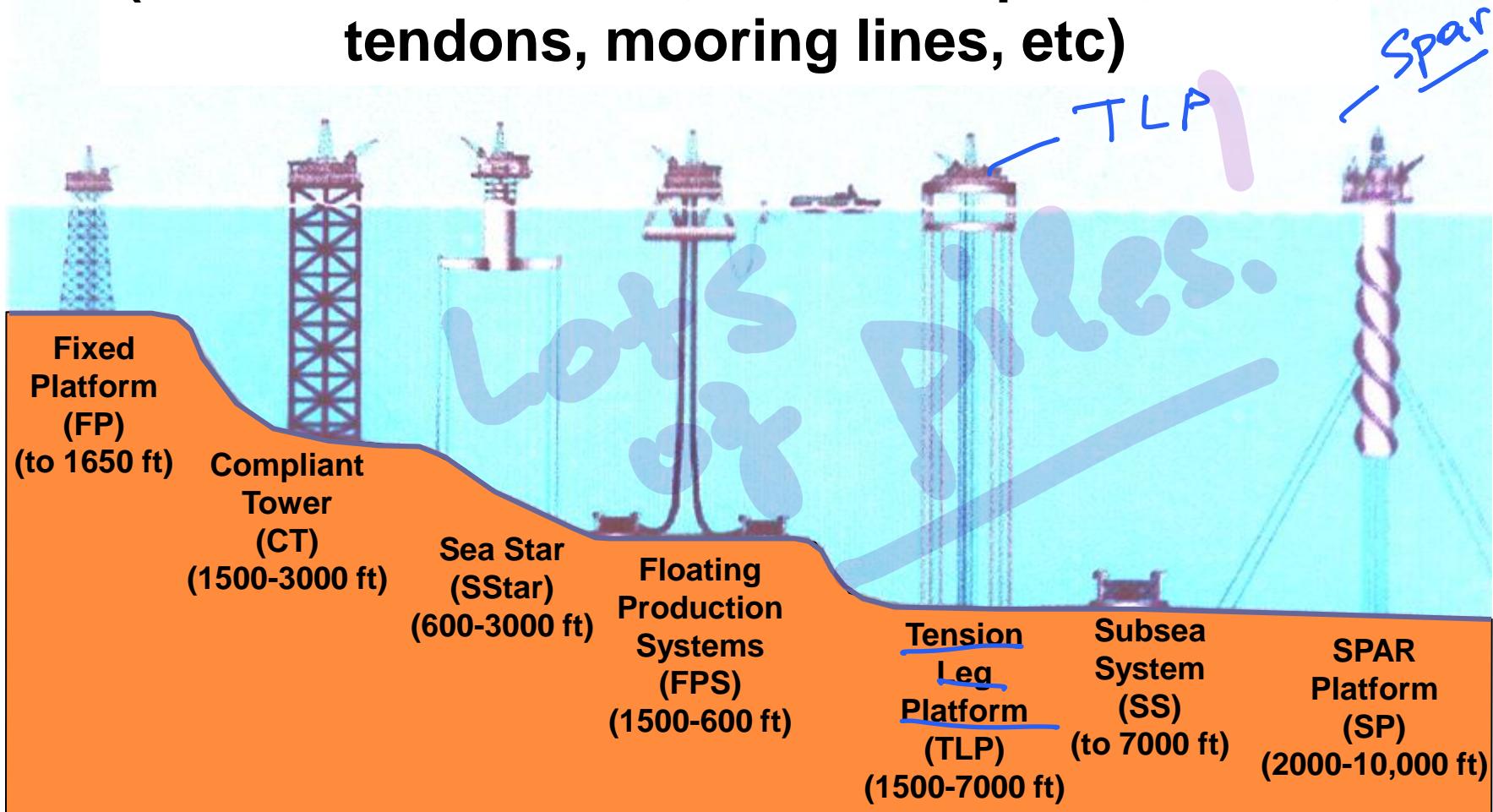
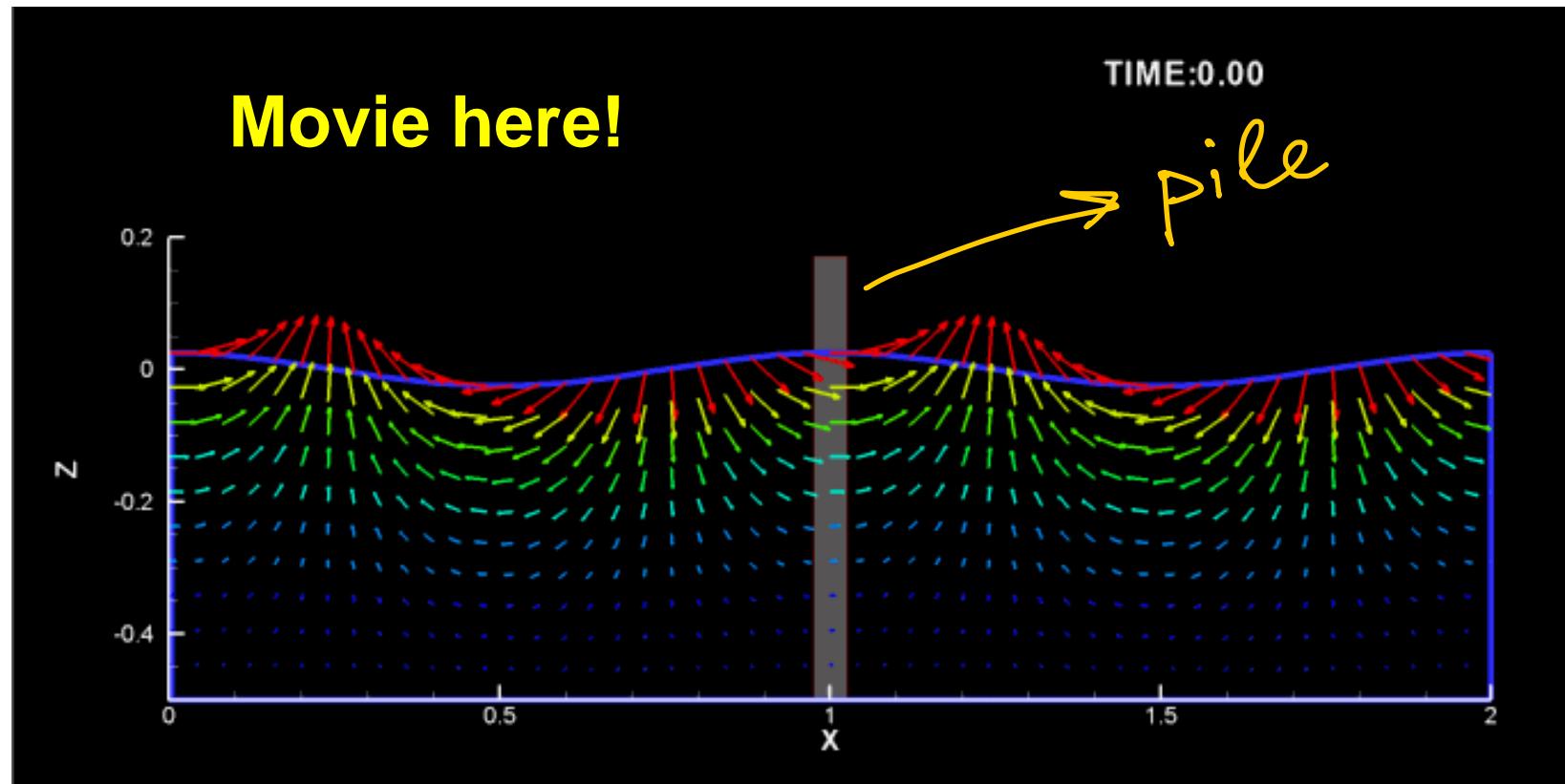
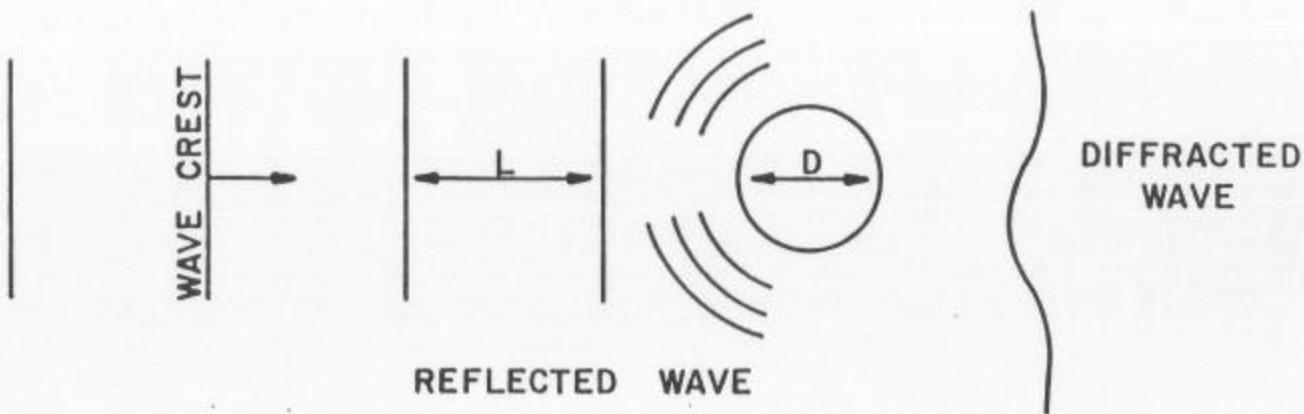


Figure from BOEMRE, U.S. Department of the Interior

What inflow velocity would a pile be subjected to?

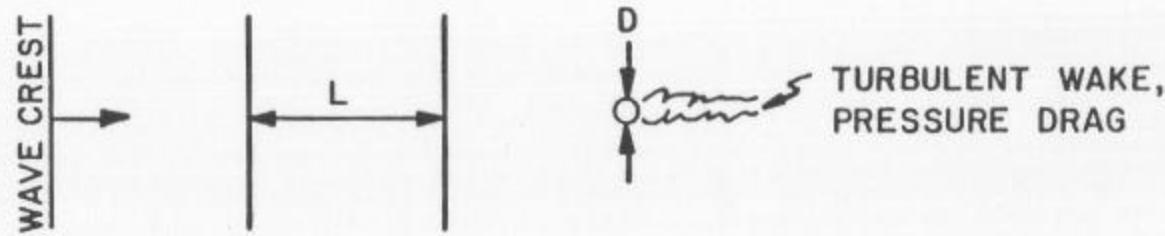


WAVE FORCE LIMITING CASE



WAVES MODIFIED BY OBJECT, $D > \frac{L}{5}$

WAVE FORCE LIMITING CASE



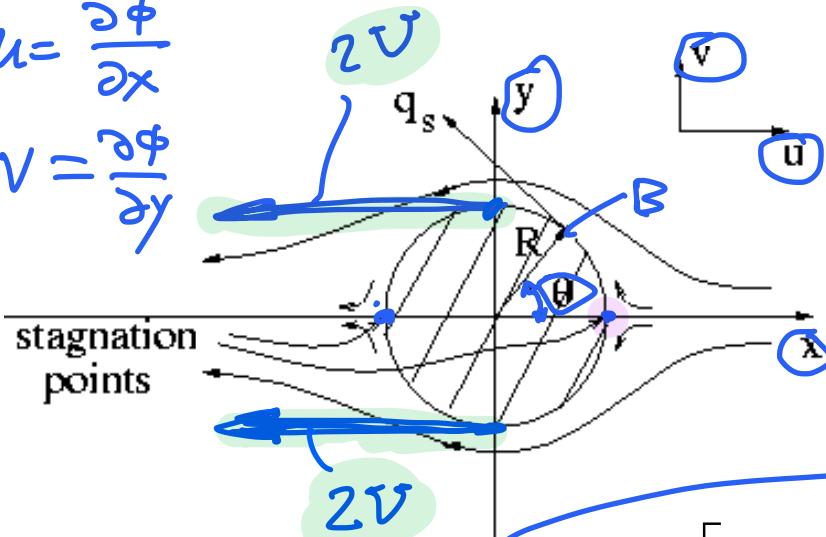
WAVE DOES NOT "FEEL" OBJECT, $D < \frac{L}{6}$

Steps to take:

- Study steady flow around 2-D cylinder (circle) subject to **steady inflow**
- Study unsteady flow around 2-D cylinder (circle) subject to **accelerating inflow**
- Apply the study and the formulas developed in the previous steps, on “slices” of the **3-D cylinder,** **subject to wave and current,** and integrate along its length to determine total forces and moments

$$u = \frac{\partial \phi}{\partial x}$$

$$v = \frac{\partial \phi}{\partial y}$$



Velocity potential:

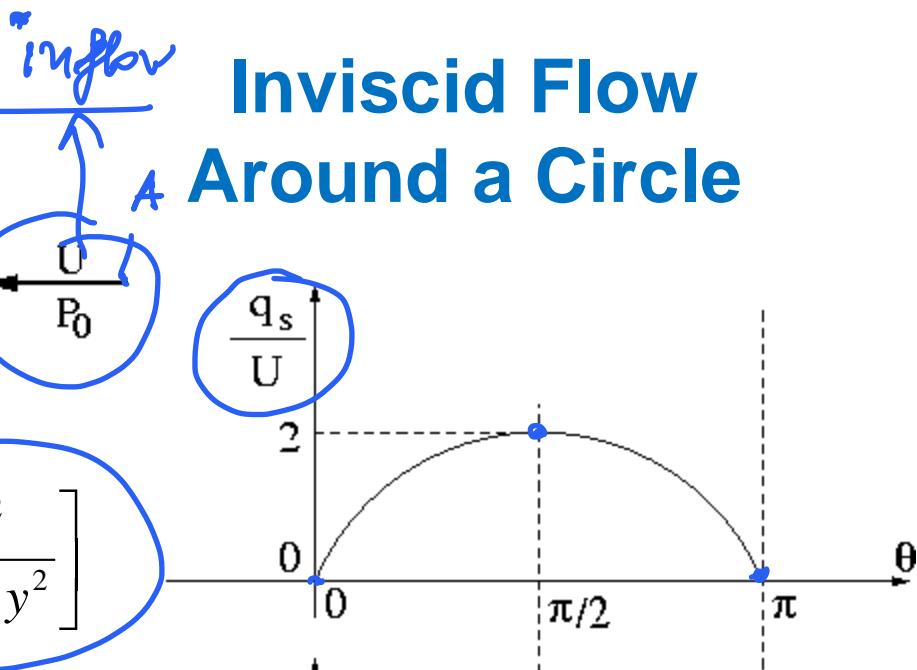
$$\Phi = -Ux \left[1 + \frac{R^2}{x^2 + y^2} \right]$$

Surface velocity

$$q_s = \sqrt{u^2 + v^2} = 2U \sin \theta$$

Surface pressure coefficient

$$C_p = \frac{P_s - P_0}{\frac{1}{2} \rho U^2} = 1 - \left(\frac{q_s}{U} \right)^2$$



Also true for general shape 3-D bodies

D'Alembert "paradox": The force on a body subject to inviscid steady flow is equal to zero!

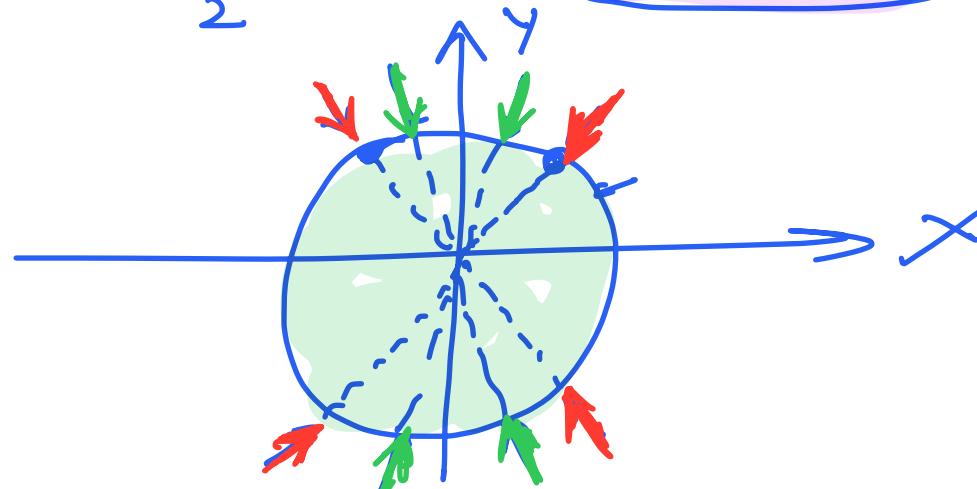
Bernoulli between \textcircled{A} far upstream
and \textcircled{B} on the surface of the circle:

$$P_A + \frac{\rho}{2} U_A^2 = P_B + \frac{\rho}{2} U_B^2$$

$$\textcircled{P_0} + \frac{\rho}{2} U^2 = \textcircled{P_B} + \frac{\rho}{2} q_s^2$$

$C_p = \text{Pressure coefficient} = \frac{P_B - P_0}{\frac{\rho}{2} U^2} =$

$$= \frac{\frac{\rho}{2} U^2 - \frac{\rho}{2} q_s^2}{\frac{\rho}{2} U^2} = \boxed{1 - \left(\frac{q_s}{U}\right)^2} \rightarrow (\text{unitless})$$



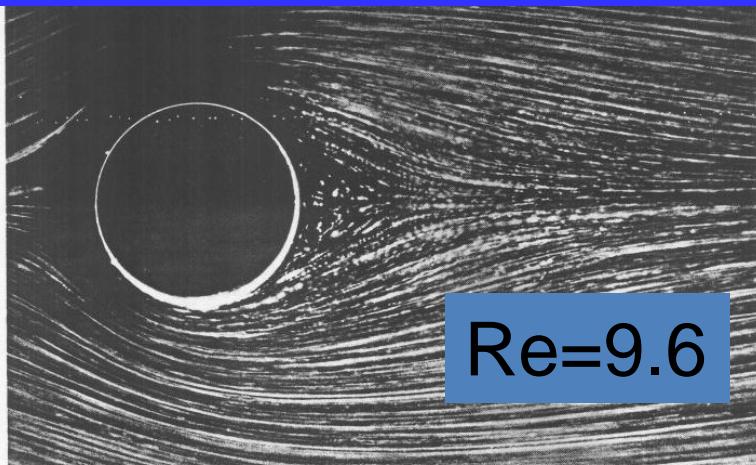
We assume
cylinder
is horizontal
 $\Delta z = 0$

$$\frac{P_B - P_0}{\frac{\rho}{2} U^2} =$$

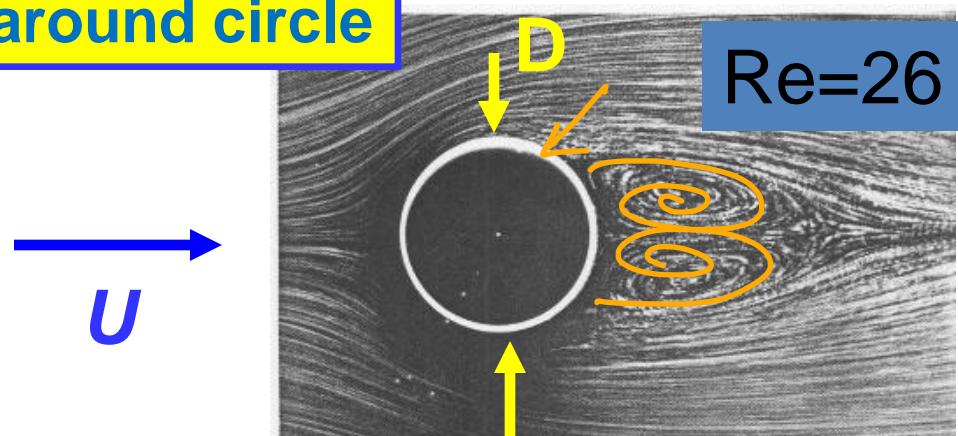
(unitless)

symmetry of
pressures
w.r.t. to x or y
results into
ZERO FORCE

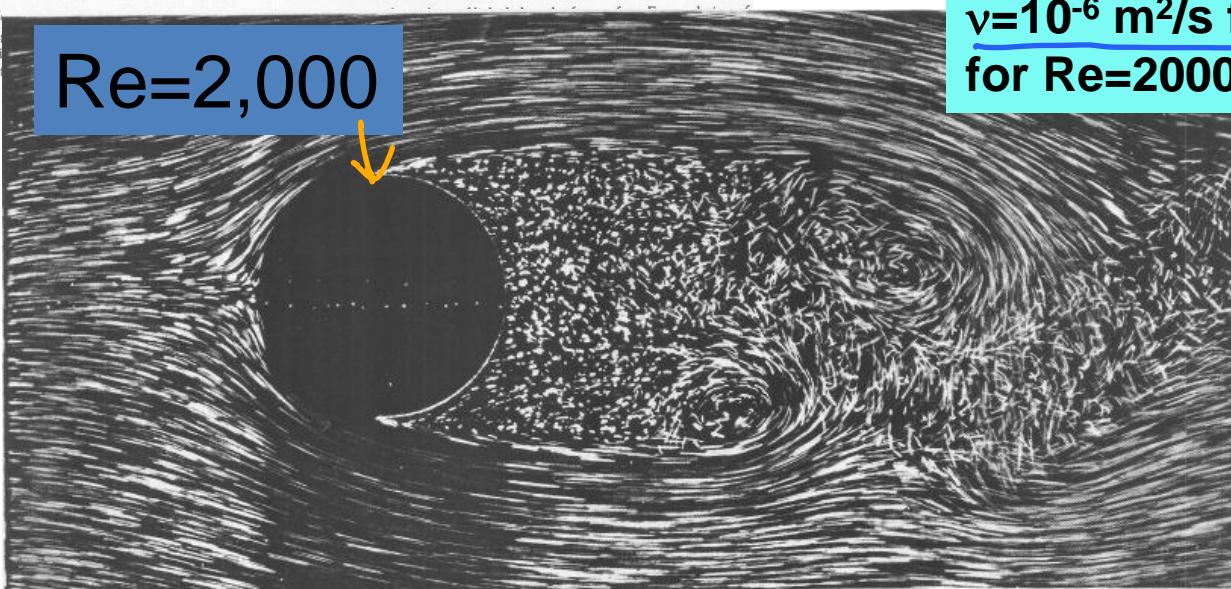
Effect of viscosity on flow around circle



Re=9.6



Re=26



Re=2,000

$\nu = \mu / \rho = \text{kinematic viscosity}$
 $\nu=10^{-6} \text{ m}^2/\text{s for H}_2\text{O at } 20^\circ \text{ C. If } D=20\text{cm}$
for Re=2000, U should be 0.01 m/s

Reynolds
Number

$$\text{Re} = \frac{UD}{\nu}$$

47. Circular cylinder at $R=2000$. At this Reynolds number one may properly speak of a boundary layer. It is laminar over the front, separates, and breaks up into a turbulent wake. The separation points, moving forward as

the Reynolds number is increased, have now attained their upstream limit, ahead of maximum thickness. Visualization is by air bubbles in water. ONERA photograph, Werlé & Gallon 1972

Photos from Album of Fluid Motion of M. Vandyke

Air

Physical properties of air and water

H₂O

Table A.3 MECHANICAL PROPERTIES OF AIR AT STANDARD ATMOSPHERIC PRESSURE

Temperature	Density	Specific Weight	Dynamic Viscosity μ	Kinematic Viscosity ν
	kg/m ³	N/m ³	N · s/m ²	m ² /s
-20°C	1.40	13.70	1.61×10^{-5}	1.16×10^{-5}
-10°C	1.34	13.20	1.67×10^{-5}	1.24×10^{-5}
0°C	1.29	12.70	1.72×10^{-5}	1.33×10^{-5}
10°C	1.25	12.20	1.76×10^{-5}	1.41×10^{-5}
20°C	1.20	11.80	1.81×10^{-5}	1.51×10^{-5}
30°C	1.17	11.40	1.86×10^{-5}	1.60×10^{-5}
40°C	1.13	11.10	1.91×10^{-5}	1.69×10^{-5}
50°C	1.09	10.70	1.95×10^{-5}	1.79×10^{-5}
60°C	1.06	10.40	2.00×10^{-5}	1.89×10^{-5}
70°C	1.03	10.10	2.04×10^{-5}	1.99×10^{-5}
80°C	1.00	9.81	2.09×10^{-5}	2.09×10^{-5}
90°C	0.97	9.54	2.13×10^{-5}	2.19×10^{-5}
100°C	0.95	9.28	2.17×10^{-5}	2.29×10^{-5}
120°C	0.90	8.82	2.26×10^{-5}	2.51×10^{-5}
140°C	0.85	8.38	2.34×10^{-5}	2.74×10^{-5}
160°C	0.81	7.99	2.42×10^{-5}	2.97×10^{-5}
180°C	0.78	7.65	2.50×10^{-5}	3.20×10^{-5}
200°C	0.75	7.32	2.57×10^{-5}	3.44×10^{-5}
	slugs/ft ³	lbf/ft ³	lbf-s/ft ²	ft ² /s
0°F	0.00269	0.0866	3.39×10^{-7}	1.26×10^{-4}
20°F	0.00257	0.0828	3.51×10^{-7}	1.37×10^{-4}
40°F	0.00247	0.0794	3.63×10^{-7}	1.47×10^{-4}
60°F	0.00237	0.0764	3.74×10^{-7}	1.58×10^{-4}
80°F	0.00228	0.0735	3.85×10^{-7}	1.69×10^{-4}
100°F	0.00220	0.0709	3.96×10^{-7}	1.80×10^{-4}
120°F	0.00213	0.0685	4.07×10^{-7}	1.91×10^{-4}
150°F	0.00202	0.0651	4.23×10^{-7}	2.09×10^{-4}
200°F	0.00187	0.0601	4.48×10^{-7}	2.40×10^{-4}
300°F	0.00162	0.0522	4.96×10^{-7}	3.05×10^{-4}
400°F	0.00143	0.0462	5.40×10^{-7}	3.77×10^{-4}

SOURCE: Reprinted with permission from R. E. Bolz and G. L. Tuve, *Handbook of Tables for Applied Engineering Science*, CRC Press, Inc., Cleveland, 1973. Copyright © 1973 by The Chemical Rubber Co., CRC Press, Inc.

Table A.5 APPROXIMATE PHYSICAL PROPERTIES OF WATER* AT ATMOSPHERIC PRESSURE

Temperature	Density	Specific Weight	Dynamic Viscosity	Kinematic Viscosity	Vapor Pressure
	kg/m ³	N/m ³	N · s/m ²	m ² /s	N/m ² abs
0°C	1000	9810	1.79×10^{-3}	1.79×10^{-6}	611
5°C	1000	9810	1.51×10^{-3}	1.51×10^{-6}	872
10°C	1000	9810	1.31×10^{-3}	1.31×10^{-6}	1,230
15°C	999	9800	1.14×10^{-3}	1.14×10^{-6}	1,700
20°C	998	9790	1.00×10^{-3}	1.00×10^{-6}	2,340
25°C	997	9781	8.91×10^{-4}	8.94×10^{-7}	3,170
30°C	996	9771	7.97×10^{-4}	8.00×10^{-7}	4,250
35°C	994	9751	7.20×10^{-4}	7.24×10^{-7}	5,630
40°C	992	9732	6.53×10^{-4}	6.58×10^{-7}	7,380
50°C	988	9693	5.47×10^{-4}	5.53×10^{-7}	12,300
60°C	983	9643	4.66×10^{-4}	4.74×10^{-7}	20,000
70°C	978	9594	4.04×10^{-4}	4.13×10^{-7}	31,200
80°C	972	9535	3.54×10^{-4}	3.64×10^{-7}	47,400
90°C	965	9467	3.15×10^{-4}	3.26×10^{-7}	70,100
100°C	958	9398	2.82×10^{-4}	2.94×10^{-7}	101,300
	slugs/ft ³	lbf/ft ³	lbf-s/ft ²	ft ² /s	psia
40°F	1.94	62.43	3.23×10^{-5}	1.66×10^{-5}	0.122
50°F	1.94	62.40	2.73×10^{-5}	1.41×10^{-5}	0.178
60°F	1.94	62.37	2.36×10^{-5}	1.22×10^{-5}	0.256
70°F	1.94	62.30	2.05×10^{-5}	1.06×10^{-5}	0.363
80°F	1.93	62.22	1.80×10^{-5}	0.930×10^{-5}	0.506
100°F	1.93	62.00	1.42×10^{-5}	0.739×10^{-5}	0.949
120°F	1.92	61.72	1.17×10^{-5}	0.609×10^{-5}	1.69
140°F	1.91	61.38	0.981×10^{-5}	0.514×10^{-5}	2.89
160°F	1.90	61.00	0.838×10^{-5}	0.442×10^{-5}	4.74
180°F	1.88	60.58	0.726×10^{-5}	0.385×10^{-5}	7.51
200°F	1.87	60.12	0.637×10^{-5}	0.341×10^{-5}	11.53
212°F	1.86	59.83	0.593×10^{-5}	0.319×10^{-5}	14.70

* Notes: (1) Bulk modulus E_v of water is approximately 2.2 GPa (3.2×10^5 psi); (2) water-air surface tension is approximately 7.3×10^{-2} N/m (5×10^{-3} lbf/ft) from 10°C to 50°C.

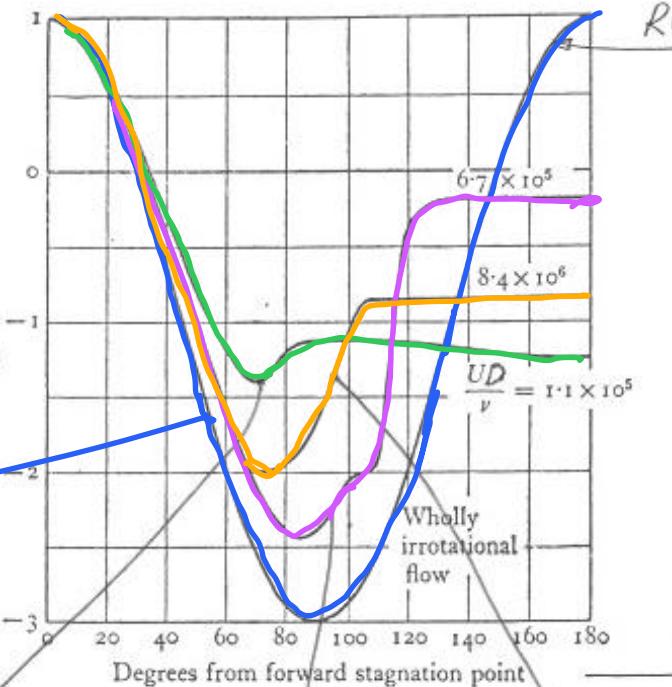
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From Engineering Fluid Mechanics of Crowe et al, 2009

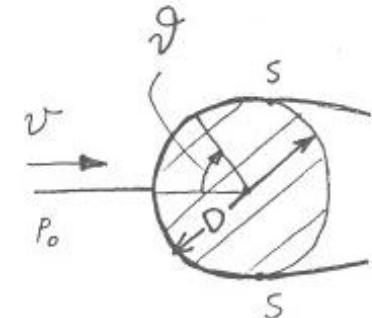
Effect of Re on the pressure distribution on surface of circle

$$C_P = \frac{p - p_0}{\frac{1}{2} \rho U^2}$$

Inviscid

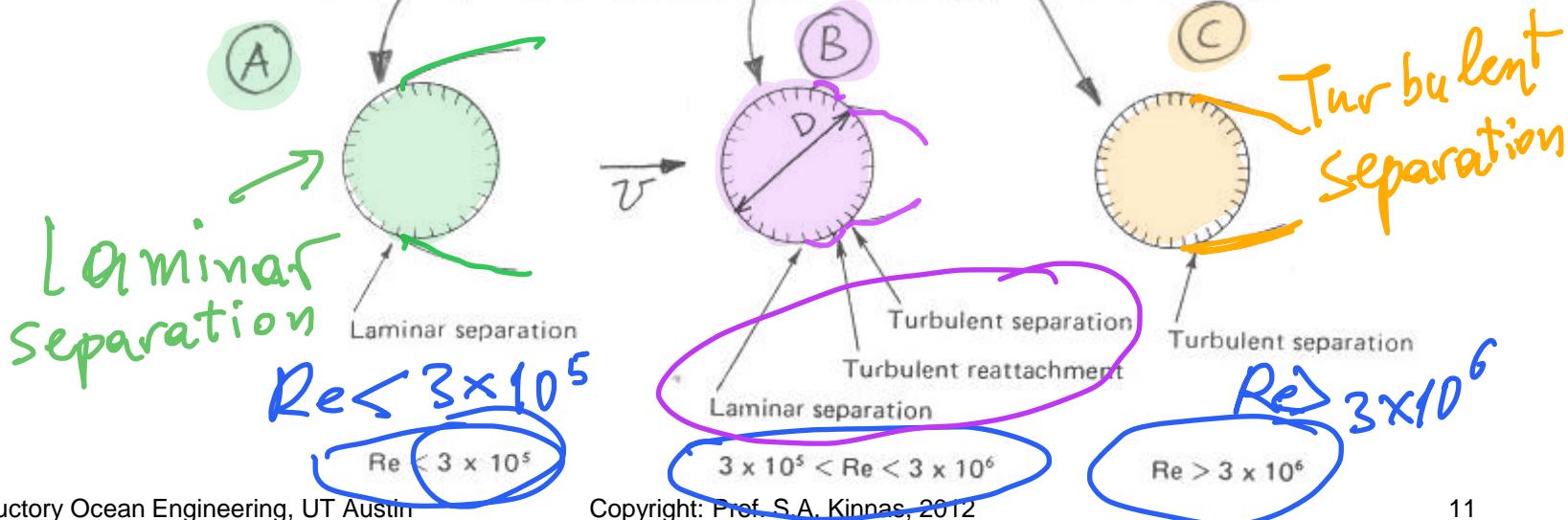


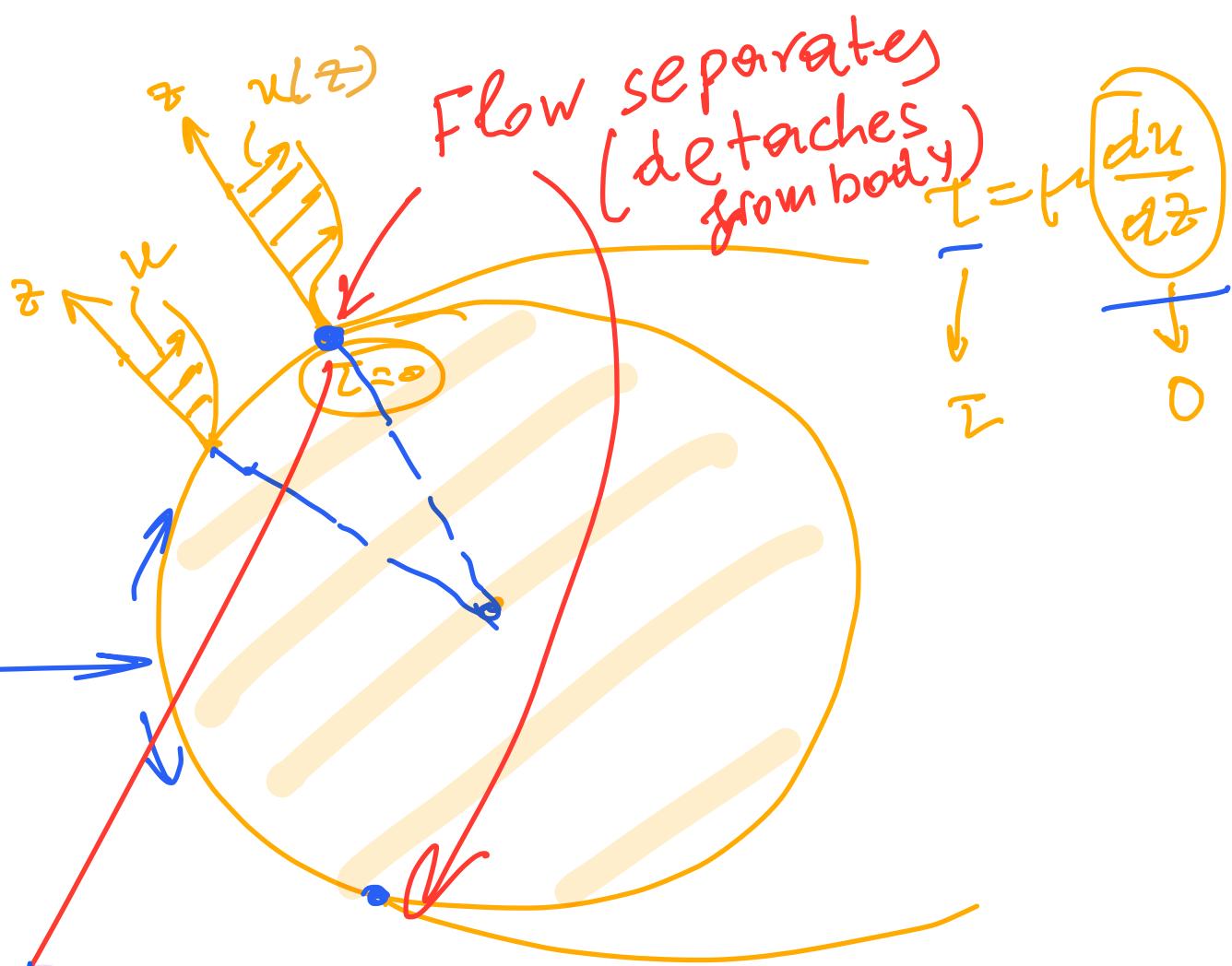
Inviscid Flow



$$Re = \frac{UD}{\nu}$$

$$\nu = \frac{\mu}{\rho} \quad (\text{Kinematic viscosity})$$





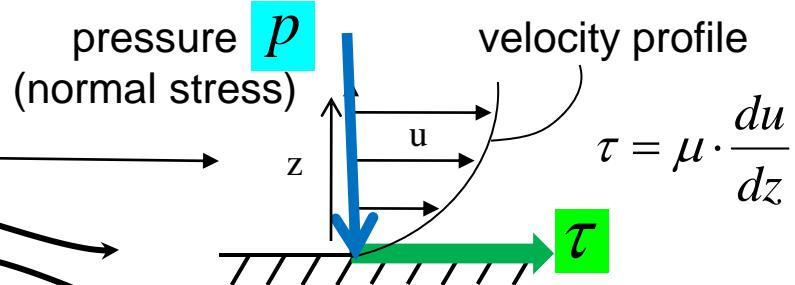
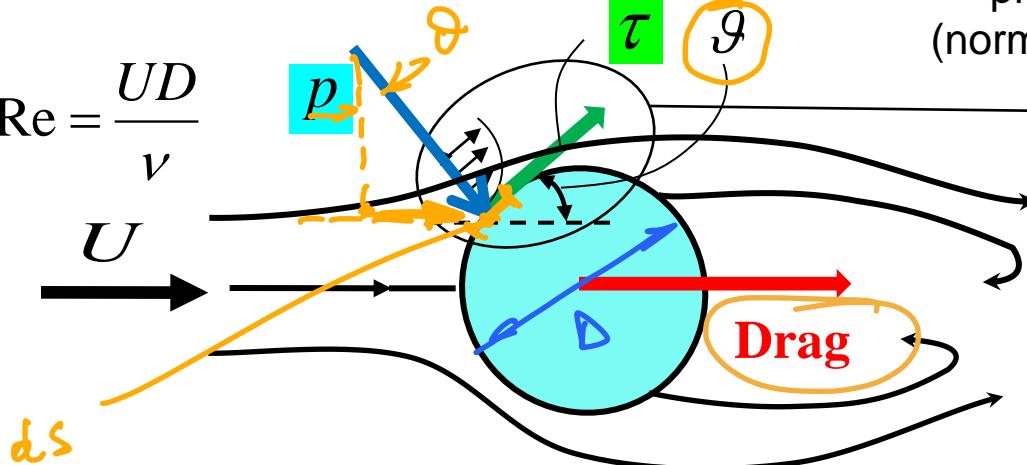
See also
 "Calculated
 Flow Around
 Cylinder in
 Uniform Flow,
 in the
 DE website!"

criterion for separation:

$$T = \mu \frac{du}{dz} = 0$$

Drag and Drag coefficient

$$Re = \frac{UD}{\nu}$$



μ =dynamic viscosity
 τ = friction (shear stress) acting on the body

Total Drag = Friction Drag + Pressure Drag

Friction Drag = $\int \tau ds \cos \theta \neq 0$
 body

Pressure (or form) Drag = $\int p ds \sin \theta \neq 0$
 body

Drag Coefficient (in 2-D):

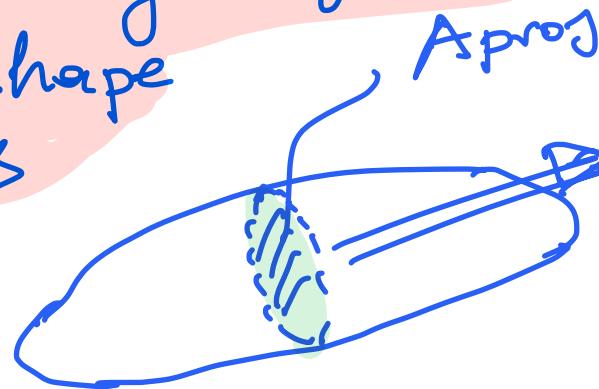
$$C_D = \frac{\text{Drag force per unit width}}{\frac{1}{2} \rho U^2 D}$$

(in 3-D):

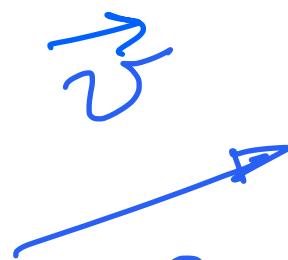
$$C_D = \frac{\text{Drag force}}{\frac{1}{2} \rho U^2 A_{proj}}$$

A_{proj} is the **projected area** of the body on a plane normal to the direction of inflow

Definition of Drag coeff. C_D
for general shape bodies



(Force in
the
same
direction
as \vec{U})



ρ
density
of fluid

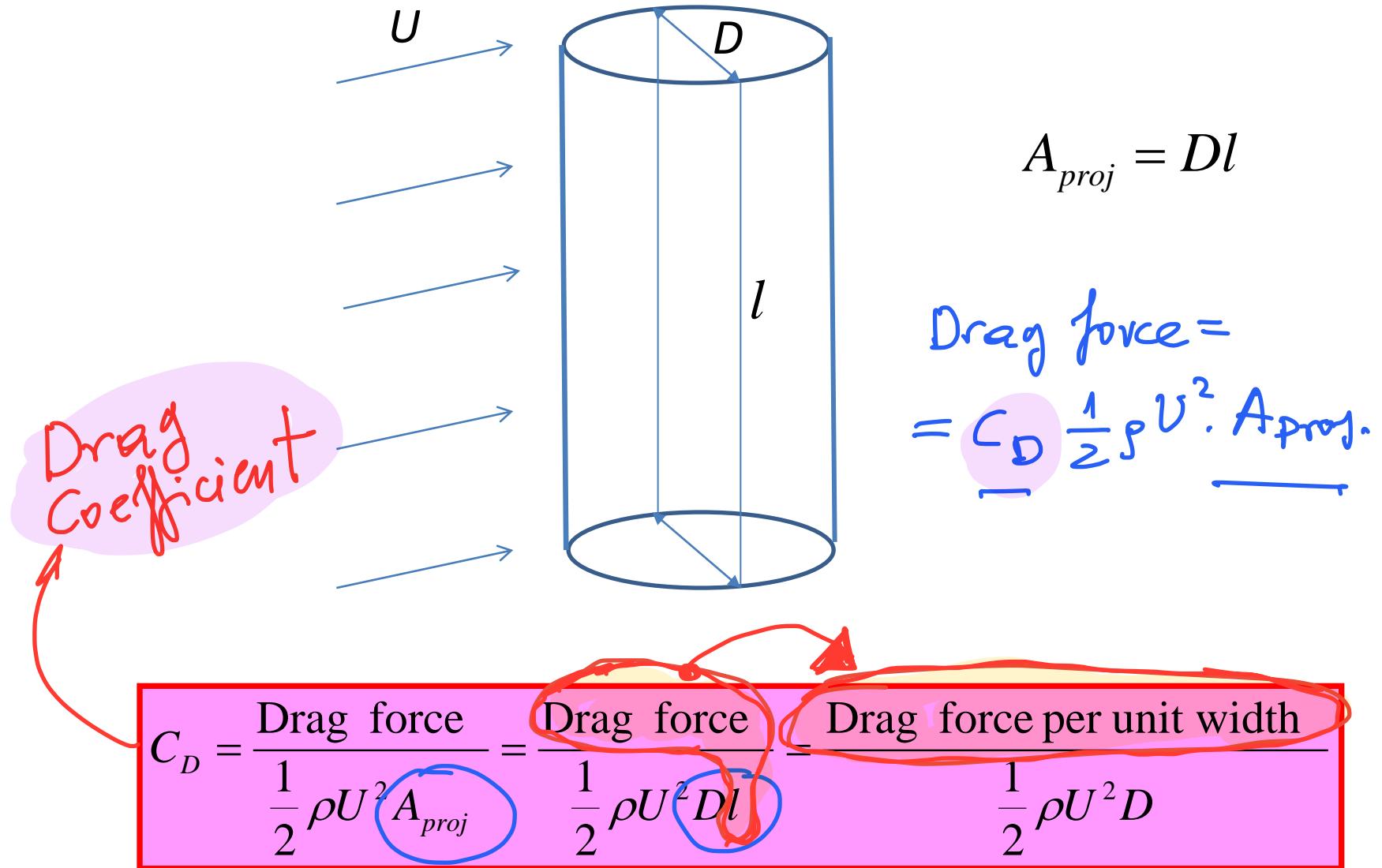
$$C_D = \frac{\text{Drag}}{\frac{1}{2} \rho U^2 A_{\text{proj.}}}$$

$A_{\text{proj.}}$ = Projected Area of object
on a plane which is
perpendicular to

* C_D is
unitless!

inflow direction \vec{U}

Drag force on a cylinder subject to uniform current U



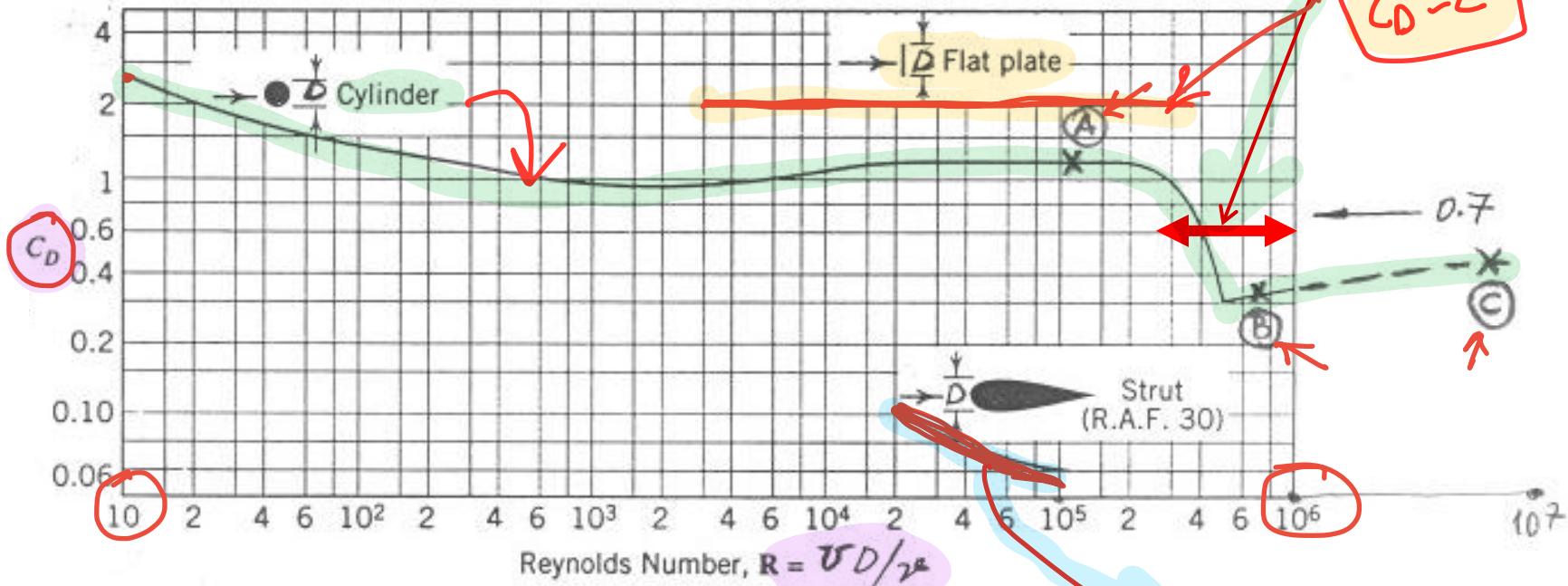
Effect of Re on Drag coefficient on Cylinder

For flat plate

Drag

Drag "crisis"

$C_D = 2$



$$C_D = \frac{\text{Drag force per unit width}}{\frac{1}{2} \rho U^2 D}$$

significantly smaller C_D for "streamlined" body

Note that in the C_D vs Re graph above:

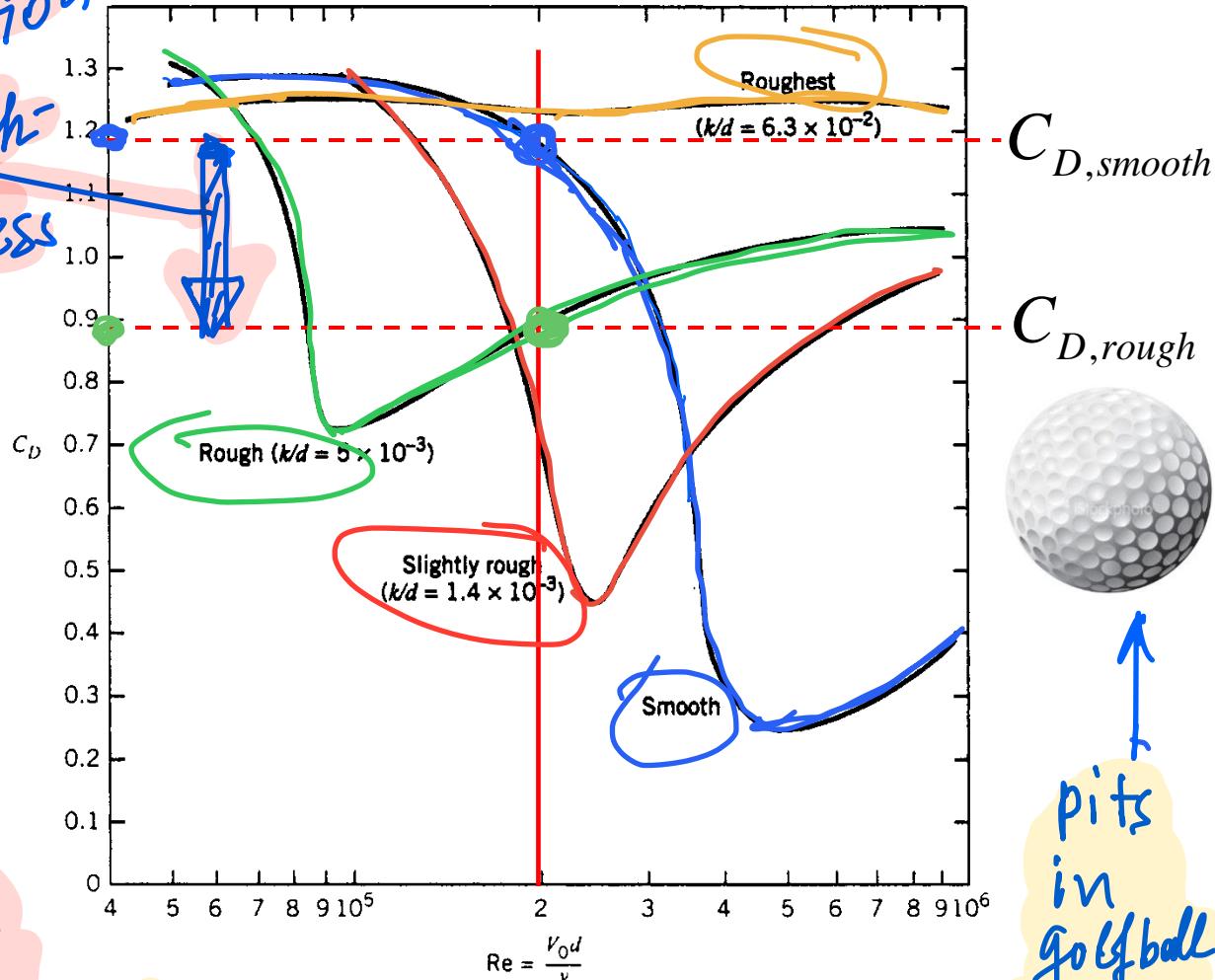
- A has the largest C_D due to larger extent of separation zone as shown in the flow pattern a few slides above
- B has the smallest C_D due to smaller extent of separation. However this condition is not recommended due to sharp change of C_D as speed/inflow changes
- C is the best choice since turbulent flow reduces separation zone $\Rightarrow C_D \downarrow$

Effect of roughness on Drag coefficient on Cylinder



$k/d =$ relative roughness

Roughness turns flow turbulent and REDUCES Drag



From Engineering Fluid Mechanics of Crowe et al, 2009

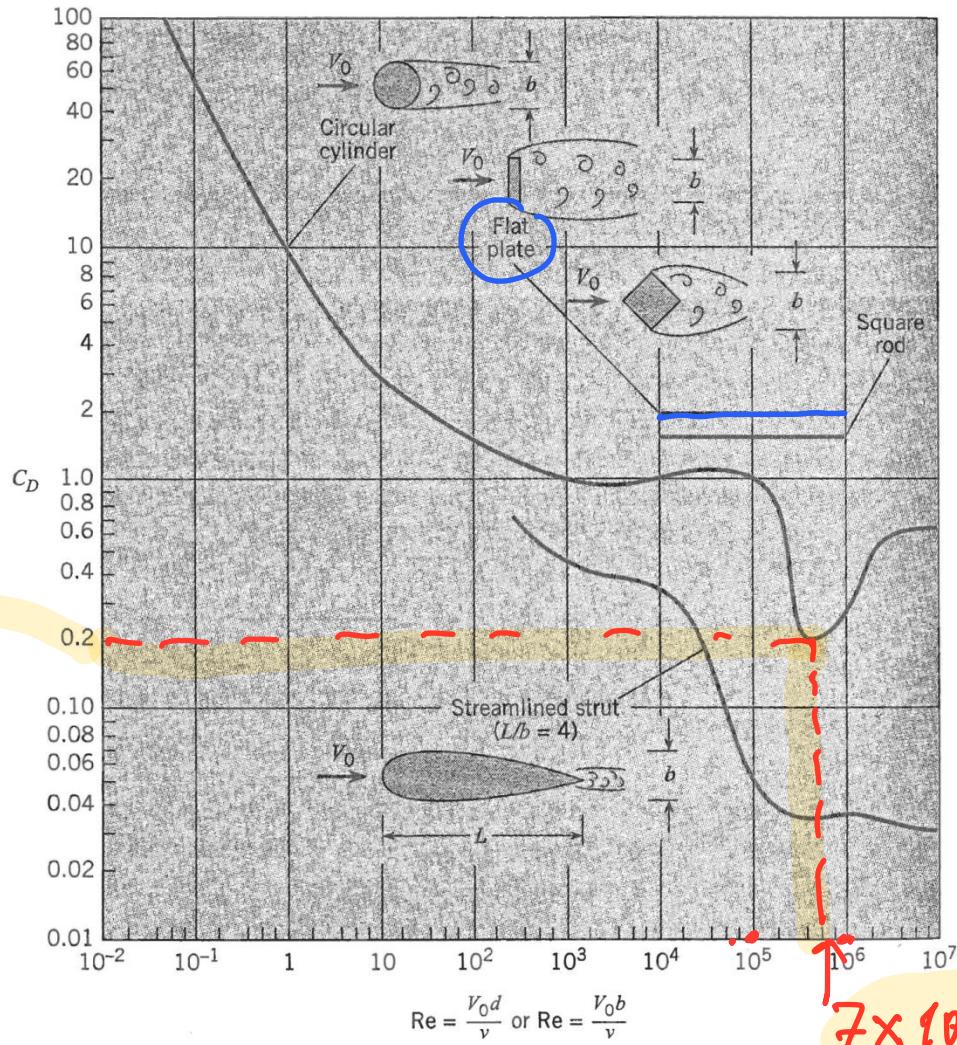
pits in golfball trigger turbulence

Drag coefficients for some other 2-D shapes

FIGURE 11.5

Coefficient of drag versus Reynolds number for two-dimensional bodies. [Data sources: Bullivant (5), Defoe (9), Goett and Bullivant (12), Jacobs (15), Jones (17), and Lindsey (21)]

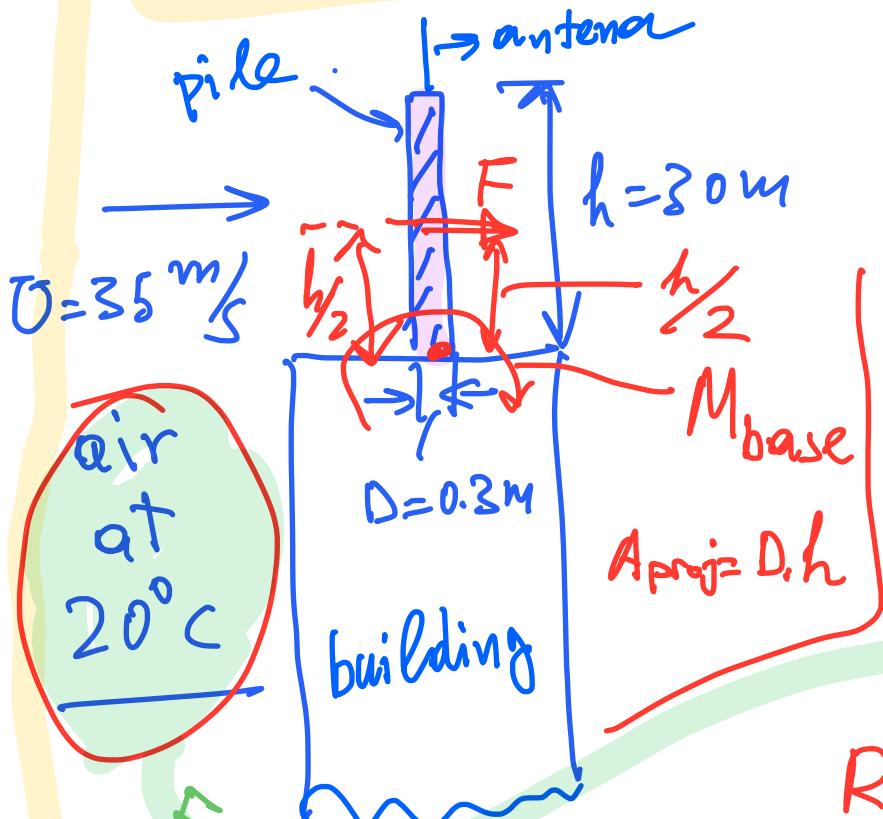
Graph used
in example
problem
in next slide



From Engineering Fluid Mechanics of Crowe et al, 2009

(excerpts are provided on class website)

Ex. 11.1 (p. 486) from Crowe's book



air at 20°C

From tables for air (see previous slides)
 $\Rightarrow C_D = 0.2$

Find Force acting on pile and moment exerted at its base.

$$F = \frac{1}{2} \rho V^2 A_{\text{proj}}$$

$$1.2 \frac{\text{kg}}{\text{m}^3}$$

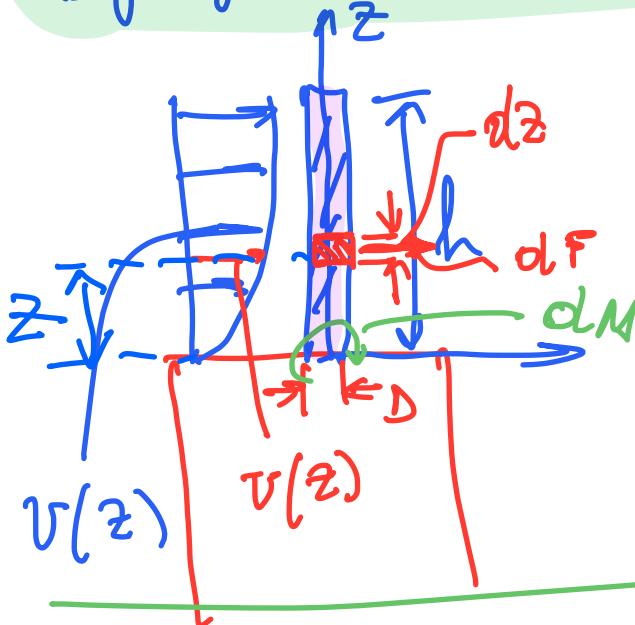
$$Re = \frac{V \cdot D}{2\eta} = \frac{35 \frac{\text{m}}{\text{s}} \times (0.3 \text{m})}{1.5 \times 10^{-5} \frac{\text{m}^2}{\text{s}}} = 7 \times 10^5$$

$$\Rightarrow F = 1,323 \text{ N}$$

Force applies at the middle of the pile
since uniform inflow is assumed.

$$M_{base} = F \times \frac{h}{2} = \underline{19,845 \text{ N-m}}$$

If flow not uniform:



$$dF = C_D \frac{1}{2} \rho [V(z)]^2 \cdot D \cdot dz$$

we assume
to be
constant

$$F = \int_0^h dF = \int_0^h C_D \frac{1}{2} \rho V(z)^2 D \cdot dz =$$

$$dM = z \cdot dF$$

$$M_{base} = \int_0^h dM = \int_0^h z \cdot dF = \int_0^h z \cdot C_D \frac{1}{2} D \rho [V(z)]^2 dz$$

$$= C_D \frac{\rho}{2} D \int_0^h z [V(z)]^2 dz$$